

GENERAL NOTES:

1. VISITS TO THE JOB SITE BY REPRESENTATIVE OF THE ENGINEER DO NOT CONSTITUTE APPROVAL OF THE WORK PERFORMED BY THE CONTRACTOR OR HIS SUBCONTRACTORS AND ARE MERELY FOR THE PURPOSE OF OBSERVING THE WORK PERFORMED.
2. CONTRACTOR SHALL NOTIFY ENGINEER / ARCHITECT OF ANY DISCREPANCIES, OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND / OR SPECIFICATIONS BEFORE PROCEEDING WITH ANY WORK. INVOLVED IN ALL CASES UNLESS OTHERWISE DIRECTED, THE MOST STRINGENT REQUIREMENTS SHALL GOVERN AND BE PERFORMED.
3. CONTRACTOR SHALL VERIFY ALL CONDITIONS, DIMENSIONS AND ELEVATIONS, ETC., AT THE SITE AND SHALL COORDINATE WORK PERFORMED BY ALL TRADES. DO NOT SCALE DRAWINGS.
4. SHOP DRAWINGS AND SUBMITTALS SHALL BE REVIEWED BY THE ENGINEER / ARCHITECT AND TENANT PRIOR TO FABRICATION OR ERECTION FOR ANY PREFABRICATED OR MANUFACTURED - DESIGNED COMPONENTS.
5. SIZES, LOCATIONS, LOADS, AND ANCHORAGE OF EQUIPMENT SHALL BE VERIFIED IN THE FIELD WITH EQUIPMENT MANUFACTURERS (SUPPLIERS) PRIOR TO FABRICATION OR INSTALLATION OF SUPPORTING STRUCTURES.
6. TEMPORARY BRACING SHALL BE PROVIDED WHEREVER NECESSARY TO TAKE CARE OF ALL LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED, INCLUDING WIND. SUCH BRACING SHALL BE LEFT IN PLACE AS LONG AS MAY BE REQUIRED FOR SAFETY, OR UNTIL ALL THE STRUCTURAL ELEMENTS ARE COMPLETED. ALL BRACING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
7. DURING AND AFTER CONSTRUCTION THE CONTRACTOR AND / OR OWNER SHALL KEEP LOADS ON THE STRUCTURE WITHIN THE LIMITS OF THE DESIGN LOAD.
8. CONTRACTOR AND ALL SUBCONTRACTORS SHALL PERFORM THEIR TRADES AND DUTIES IN A MANNER CONFORMING TO THE PROCEDURES AND REQUIREMENTS AS STATED IN THE 2006 INTERNATIONAL BUILDING CODE, (OR LATEST ACCEPTED CODE ADOPTED BY THE LOCAL BUILDING OFFICIALS), AND LOCAL ORDINANCES.
9. ANY SPECIAL INSPECTION REQUIRED BY THE BUILDING OFFICIAL OR THE INTERNATIONAL CODE ARE THE RESPONSIBILITY OF THE OWNER. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING SPECIAL INSPECTIONS AND TESTS ARE PERFORMED PRIOR TO PROCEEDING WITH THE WORK.
10. CONTRACTOR SHALL BE RESPONSIBLE FOR SAFETY AND PROTECTION WITHIN AND ADJACENT TO THE JOB SITE.
11. ALL PLUMBING WORK SHALL BE IN STRICT ACCORDANCE WITH THE LATEST EDITION OF THE INTERNATIONAL PLUMBING CODE, AND LOCAL ORDINANCES. ALL PLUMBING WORK AND FIXTURES MUST MEET THE APPROVAL OF THE OWNER, CONTRACTOR, ARCHITECT/ENGINEER, TENANT AND THE BUILDING OFFICIAL.
12. ALL HVAC WORK SHALL BE IN STRICT ACCORDANCE WITH THE LATEST EDITION OF THE INTERNATIONAL MECHANICAL CODE, AND LOCAL ORDINANCES. HVAC WORK, UNITS, AND CONTROLS, MUST MEET THE APPROVAL OF THE OWNER, CONTRACTOR, ARCHITECT/ENGINEER, TENANT, AND THE BUILDING OFFICIAL.
13. CONSTRUCTION MUST BE IN COMPLIANCE WITH THE INTERNATIONAL ENERGY CONSERVATION CODE.
14. CONSTRUCTION MUST BE IN COMPLIANCE WITH THE CURRENT INTERNATIONAL FIRE CODE.
15. ALL ELECTRICAL WORK SHALL BE IN STRICT ACCORDANCE WITH THE LATEST EDITION OF THE ICC ELECTRICAL CODE AND LOCAL ORDINANCES. ALL ELECTRICAL WORK, FIXTURES, SWITCHES, ETC... MUST MEET APPROVAL OF THE OWNER, CONTRACTOR, ARCHITECT / ENGINEER, TENANT AND BUILDING OFFICIAL.
16. REST ROOMS, ETC... SHALL COMPLY WITH THE LATEST ADA REQUIREMENTS, NATIONAL AND LOCAL.
17. ALL WORK MUST MEET THE APPROVAL OF THE BUILDING OWNERS, THE TENANT, THE DESIGNER, AND THE BUILDING AND ZONING DEPARTMENTS.
18. ALL FURNITURE, PLANTS, INTERIOR SIGNAGE, FILES / FILING CABINETS, APPLIANCES, OFFICE EQUIPMENT TO BE FURNISHED, INSTALLED AND PAID FOR BY THE TENANT.
19. ANY AND ALL CHANGES OR VARIATIONS FROM THESE DOCUMENTS MUST BE APPROVED IN WRITING PRIOR TO MAKING THEM.

1
A1.1

SITE PLAN

SCALE (24x36): 1:30 = 1'-0"
SCALE (11x17):



aeurbia
architects and engineers
909 W. South Jordan Parkway Unit 84095
Phone: 801.746.0456 Web: Page@aeurbia.com



PETERBILT KELSO REMODEL
2408 TALLEY WAY
KELSO, WA

AE2022.314

SITE PLAN

REVISIONS:

- - -

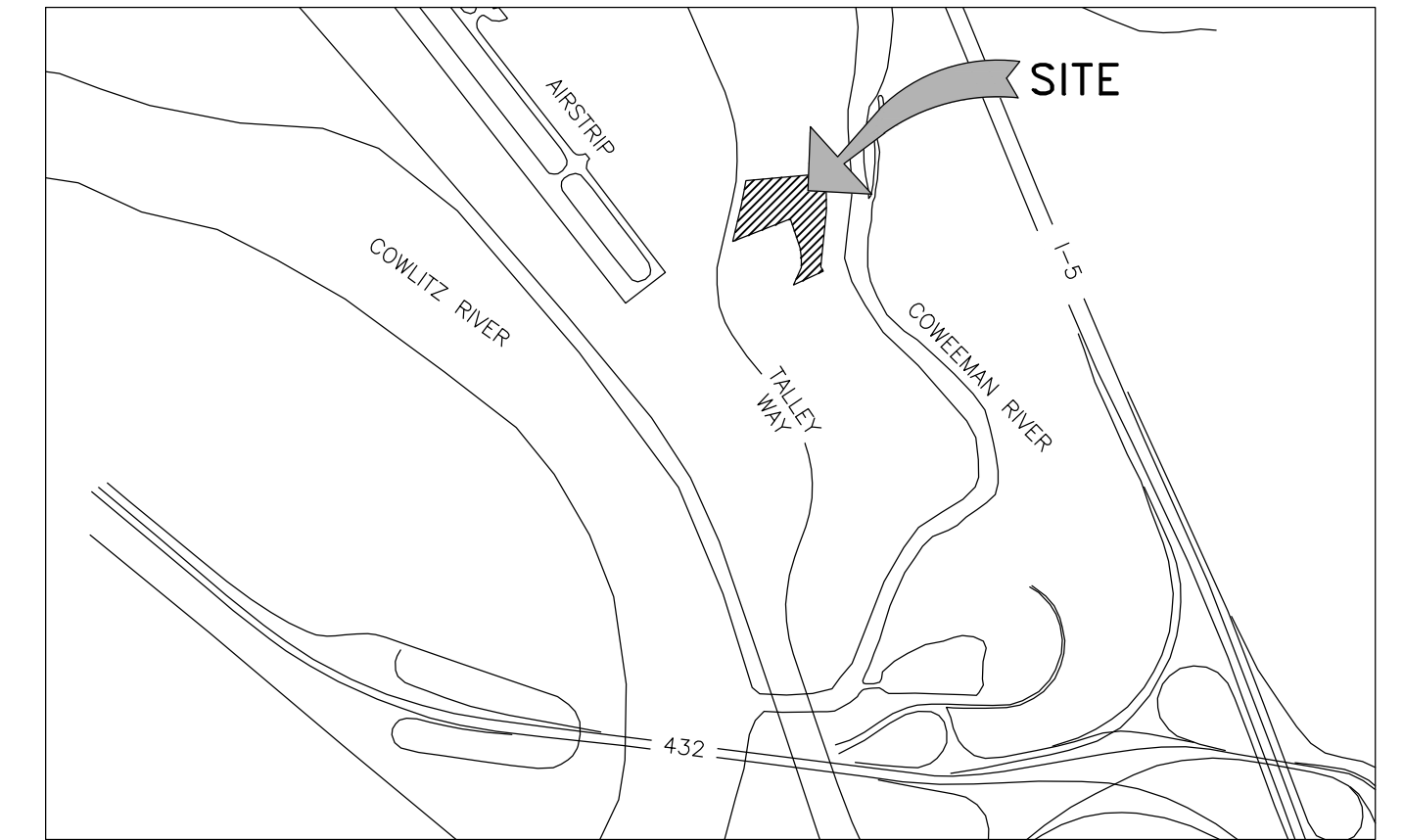
DATE: 8/8/22

SHEET NO.

A1.1

TOPOGRAPHIC SURVEY

LOCATED IN THE SOUTH HALF OF SECTION 07, AND THE NORTH HALF OF SECTION 2
TOWNSHIP 7 SOUTH, RANGE 2 WEST, W.M.
CITY OF KELSO, COWLITZ COUNTY,
WASHINGTON

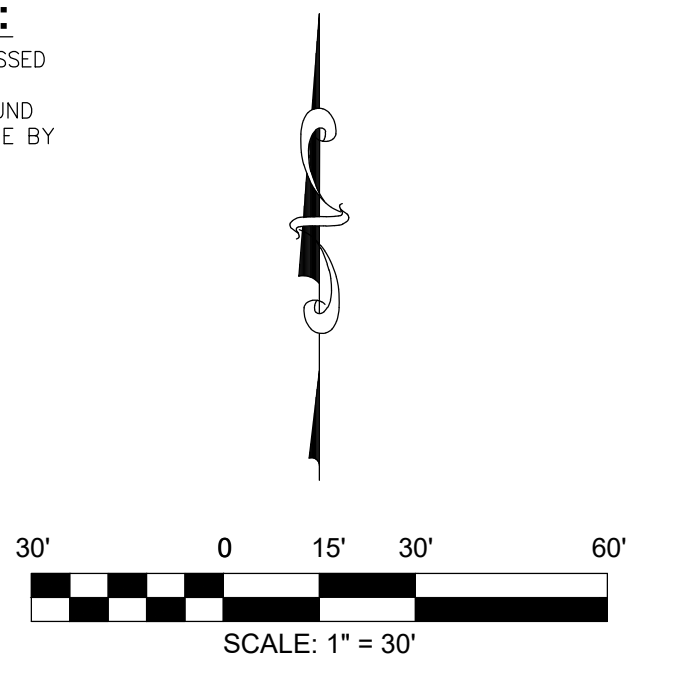


VICINITY MAP
CITY OF KELSO, COWLITZ COUNTY, WASHINGTON



HORIZONTAL DATUM (BASIS OF BEARINGS):
HORIZONTAL DATUM: NAD83(2011) BASED ON STATIC GPS OBSERVATIONS ON POINT 1, PROCESSED THROUGH TRIMBLE RTX
PROJECTION: WASHINGTON STATE PLANE COORDINATE SYSTEM, SOUTH ZONE, SCALED TO GROUND AROUND POINT 1 WITH A SCALE FACTOR OF 1.0000492330. TO OBTAIN GRID DISTANCES, DIVIDE BY THE SCALE FACTOR.
UNITS: US SURVEY FEET.

VERTICAL DATUM:
VERTICAL DATUM: NAVD88 BASED ON STATIC GPS OBSERVATIONS ON POINT 1, PROCESSED THROUGH TRIMBLE RTX.



LEGEND:

- FOUND MONUMENT
- SANITARY SEWER MANHOLE
- STORM CATCH BASIN
- ⊙ UNKNOWN MANHOLE
- UNKNOWN CLEANOUT
- UNKNOWN VAULT
- ⊙ POWER METER
- ⊙ POWER POLE
- ⊙ POWER POLE W/ LIGHT
- ⊙ LIGHT-LAMP POST
- ⊙ POWER RISER
- ⊙ WATER METER
- ⊙ WATER VALVE
- ⊙ TELEPHONE MANHOLE
- ⊙ TELEPHONE RISER
- ⊙ CATV RISER
- ⊙ GAS METER
- FP FLAG POLE
- MB MAILBOX
- BOLLARD
- ♿ HANDICAP PARKING
- ▭ BUILDING
- ▭ BUILDING OVERHANG
- ▭ PARKING STRIPES
- ▭ EDGE OF PAVEMENT
- ▭ EDGE OF CONCRETE
- ▭ EDGE OF GRAVEL
- ▭ STORM SEWER
- ▭ GAS
- ▭ OVERHEAD POWER
- ▭ FIBER OPTIC
- ▭ FENCE - AS NOTED
- ▭ BOUNDARY LINE
- ▭ EASEMENT
- ▭ LOT/PARCEL LINE
- ▭ RIGHT OF WAY

NOTES:

- THE LOCATION OF EXISTING UNDERGROUND UTILITY FACILITIES SHOWN HEREON ARE BASED ON LOCATE MARKS REQUESTED FOR THIS SURVEY PER WASHINGTON ONE CALL PUBLIC LOCATE TICKET NO. 22184409. THE SURVEYOR ASSUMES NO RESPONSIBILITY FOR THE ACCURACY OF THE DELINEATION OF SUCH UNDERGROUND UTILITIES BY THE RESPECTIVE UTILITY OWNERS, NOR FOR THE EXISTENCE OF BURIED OBJECTS WHICH ARE NOT SHOWN ON THE SURVEY. ALL UTILITY LOCATIONS SHOULD BE FIELD VERIFIED PRIOR TO CONSTRUCTION.
- FIELD WORK WAS COMPLETED ON MAY 10, 2022.
- EASEMENTS SHOWN HEREON WERE CALCULATED FROM COWLITZ COUNTY SURVEY IN BOOK 2, PAGE 197, AND BOOK 15, PAGE 197. NEITHER S&F LAND SERVICES NOR A LOCAL TITLE COMPANY HAVE PERFORMED RESEARCH FOR EXISTING EASEMENTS OTHER THAN THOSE SHOWN ON SAID SURVEYS.
- EXISTING PARCEL IDENTIFICATION NUMBERS ARE FROM COWLITZ COUNTY GIS AND ARE USED FOR INFORMATIONAL PURPOSES ONLY.
- BOUNDARY LINES ARE CALCULATED FROM RECORD INFORMATION IN SURVEY BOOK 2, PAGE 197 AND SURVEY BOOK 15, PAGE 197, AND FOUND MONUMENTS. MONUMENTS OF RECORD WERE NOT FOUND AT THE CORNERS OF THE SUBJECT PROPERTY.

S&F Land Services
PORTLAND, VANCOUVER, BEND, SEASIDE
400 E EVERGREEN BLVD.
STE 1, VANCOUVER, WA 98660
(360) 326-2334

SURVEY FOR:
**ULTIMATE STEEL
ERECTION**

AUDITOR'S FILE NUMBER 3692420
SW 1/4 OF SECTION 2
T. 7 N., R. 2 W., W.M.
COWLITZ COUNTY, WASHINGTON

WWW.SFLANDS.COM	DATE	JOB NO.	FIELD	EMAIL: INFO@SFLANDS.COM	CHECKED
	MAY 16, 2022	2022-507-01	TD	REB	AJP

GENERAL STRUCTURAL NOTES (DRAWING NOTES)

A. CODES AND SPECIFICATIONS

- International Building Code (IBC) - 2018 Edition
- ACI 318-14 Building Code Requirements for Reinforced Concrete
- AISC - Manual of Steel Construction - 15TH Edition ASD
- AISI S202-15 Code of Standard Practice for Cold-Formed Steel Structural Framing, 2015 Ed

B. DESIGN LOADS UNIFORM (PSF)

- Roof Loads
 - Live Load 20 PSF
 - Snow Load 25.0 PSF
 - Design Snow Load
 - Flat Roof Snow Load $P_f = 21.0$
 - $C_e = 1.0; C_t = 1.20; I_s = 1.0$
 - Ground Snow Load 25.0
 - Sloped Roof $P_s = 21.0; C_s = 1.0$
- Rain Load = N/A

2. Lateral Loads

- Wind Load 115 MPH (3 Sec Gust) Risk Cat= II; EXP = C
 - Encl. Cat. - Open Building
 - Internal Pressure Coef. +/- 0.00
 - Components & Cladding - 15.0 PSF
- Seismic Load: Risk Cat: II Importance Factor = 1.0
 - $S_s = 88.10\% G; S_1 = 42.50\% G$
 - Site Class = D
 - $S_{ds} = 0.674; S_{d1} = 0.531$
 - Seismic Design Category = D
 - Basic Seismic System = Bearing wall System
 - Design Base Shear $V = 0.193 W$
 - Seismic Response Coef. $C_s = 0.193$
 - Response Modification Factor $R = 6.5$
 - Analysis Procedure = Equivalent Lateral - Force Analysis

C. FOUNDATIONS

- Bearing pressure taken as 1500 PSF for column and wall footings, based on a sand, silty sand, clayey sand, silty gravel, and clay gravel (Table 1806.2, Class of Materials #4). Notify Engineer if conditions encountered are different.
- Bear footing on same type of undisturbed soil or rock throughout the entire structure.

D. MATERIALS

- Concrete f_c - Flgs 3500 PSI Exposure Class = F1 Air Content 5%
 - Max Water-Cement Ratio 0.55
 - Max Aggregate size 3/4"
- Reinforcing Steel ASTM A615 - Grade 60
- Anchor Bolts A36 / F1554 GRADE 36
- Structural Steel
 - W Sections ASTM A992 Grade 50
 - Misc. Steel (Plates, Angles, Channels, ETC.) A36 $F_y = 36$ KSI
 - Structural Tube Steel ASTM A500 Grade C ($F_y = 50$ ksi)

E. REINFORCED CONCRETE

- Concrete shall be of ready mix type conforming to ASTM C94.
- Portland Cement to comply with ASTM C150
- Comply with ACI 306 Cold Weather Concrete standards if the mean daily temperature is expected to drop below 40° F for 3 or more successive days. Place no concrete against frozen earth.
- Comply with ACI 305 Hot Weather Concrete Standards.
- Conduct all compression tests according to ACI Standard Recommended Practice for Evaluation of Compression Test Results of Field Concrete (ACI 214)
- Control joints in large areas of slab on grade shall be placed in checkerboard fashion in lengths not to exceed 20 feet in any direction.
- All construction joints shall be located so as not to impair the strength of the structure. Unless noted on the drawings, all reinforcement shall be continuous through the joints. Each construction joint shall be keyed.
- No aluminum products shall be embedded in the concrete. Electrical conduit shall be placed at mid-height of the slabs and shall have an O.D. less than one-third the slab thickness.
- Add 2-#5 reinforcing bars around all opening (unless noted otherwise) and extend 24" beyond the corner of the openings.

F. REINFORCING STEEL

- All detailing, fabrication and placing of reinforcing bars shall conform to the ACI Manual of Standard Practice for Detailing Reinforced Concrete Structures (ACI 315). All reinforcement to be supported in the forms and space with wire or plastic bar supports. Reinforcement in footings shall be supported on precast concrete block supports conforming to the Concrete Reinforcing Steel Institute Manual of Standard Practice.
- Splice of reinforcement at points of maximum stress shall be avoided wherever possible. See Table.
- All continuous reinforcement shall terminate with 90 degree return or hook or separate corner bar.
- All vertical reinforcement in columns and walls shall be doweled from the footing or structure below with rebar of the same size and spacing as required above.
- Minimum concrete cover for reinforcing bars shall be as specified in Building Code Requirements for Reinforced Concrete (ACI 318)
- Welding or tack welding of reinforcing bars is prohibited unless specifically approved by the Engineer.

G. STRUCTURAL STEEL

- Comply with the AISC and the AWS Standard code for Arc and Gas Welding in Building Construction.
- All field welders shall be qualified by test per American Welding Society standard qualifications procedure to perform the work required.
- All welds shall be made using E70XX electrodes.
- All steel members shall be given one shop coat of approved paint. Surfaces to be embedded in concrete shall not be painted.

H. COLD FORMED STEEL

- Metal floor deck shall be formed from sheets conforming to ASTM A611 or A446, Grade A or higher.
- Decking shall be furnished in the longest practical lengths and shall span continuously over not less than three spans, unless noted otherwise.

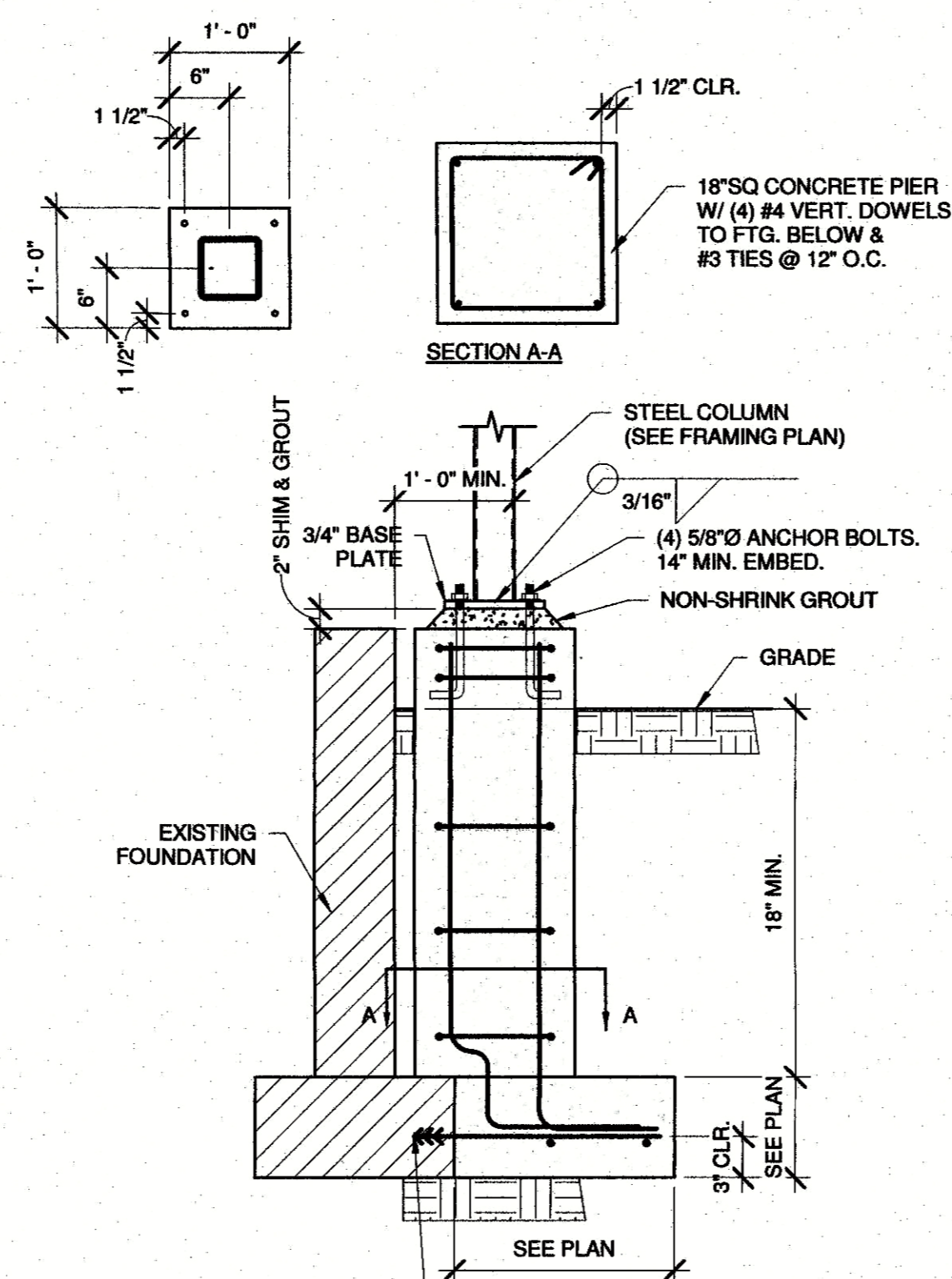
I. DEFERRED SUBMITTALS

- List of deferred submittals (shop drawings) that require Architectural and/or Engineering review and approval before fabrication or installation begins:
- Steel Shop Drawings

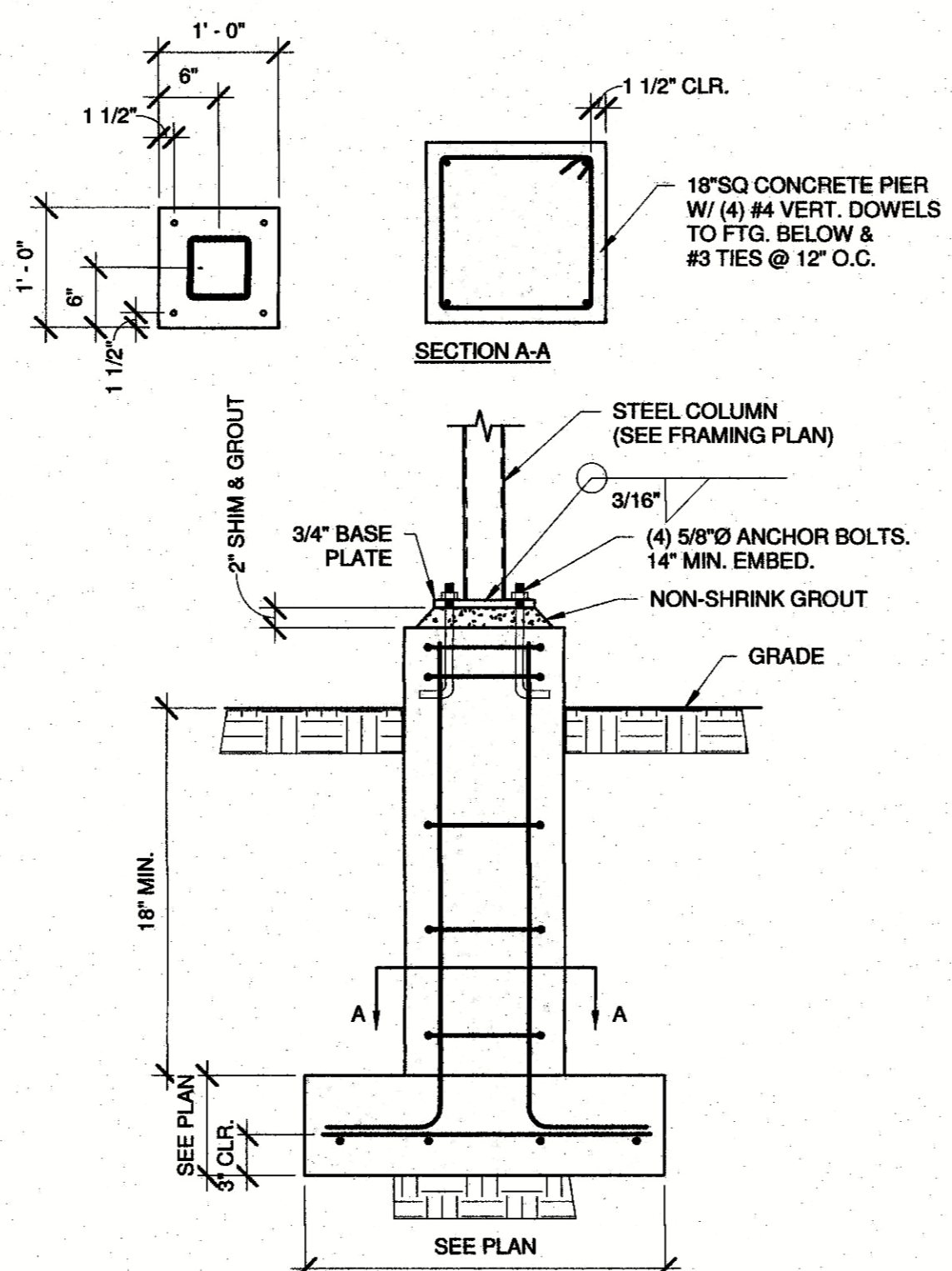
GENERAL NOTES

- Adequate shoring and bracing of all structural members during construction shall be provided.
- Any proposed field changes shall have prior approval from the Engineer.
- Contractor shall verify all dimensions in the field. Any variation from the drawings shall be brought to the attention of the Engineer.
- Install Simpson hardware as per Manufacturers requirements.
- Details are representations/depictions only. Follow written callouts.
- NO CHANGES PERMITTED WITHOUT EXPRESS WRITTEN PERMISSION OF ENGINEER

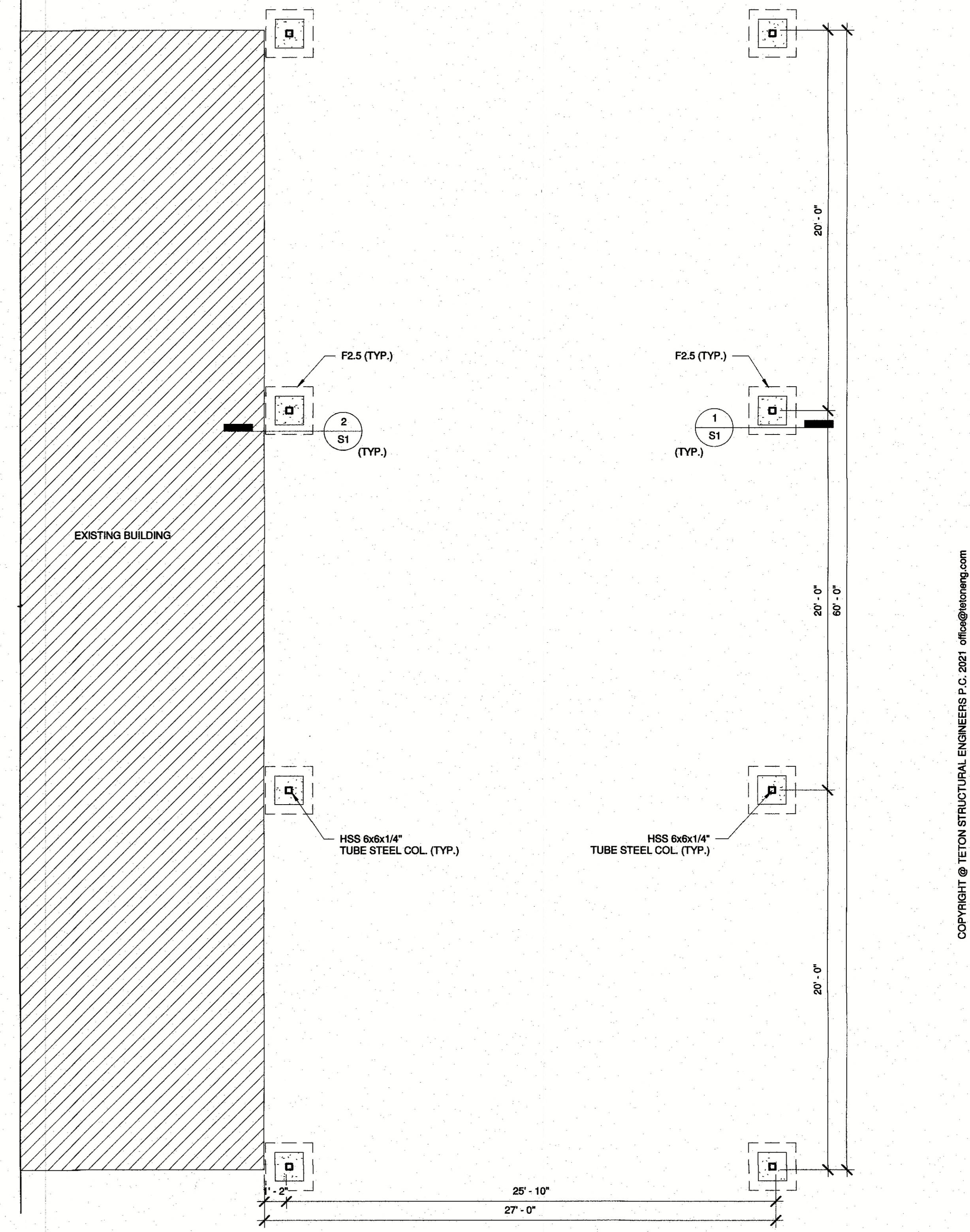
WIND COMPONENT AND CLADDING SCHEDULE			
WIND PRESSURE (PSF)	20 SF	50 SF	100 SF
ROOF AT LEAST 3'-0" AWAY FROM ANY EDGE	-10.0, -23.6	-10.0, -21.2	-10.0, -19.3
ROOF WITHIN 3'-0" OF ANY EDGE	-10.0, -43.2	-10.0, -36.9	-10.0, -32.1
WALL AT LEAST 3'-0" AWAY FROM ANY CORNER	+13.5, -13.5	+12.2, -12.6	+11.2, -12.0
WALL WITHIN 3'-0" OF ANY CORNER	+13.5, -27.0	+12.2, -23.6	+11.2, -21.0



SECTION A-A DO NOT SCALE



SECTION A-A DO NOT SCALE

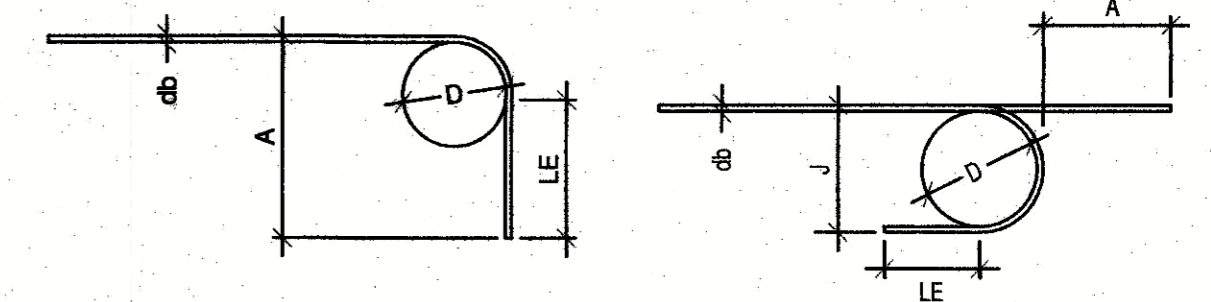


FOUNDATION PLAN 1/4" = 1'-0"

REBAR LAP SPLICE SCHEDULE		
BAR SIZE	min. $f_c = 3000$ PSI	TYP. SPLICES
NO.	DIA.	LENGTH (L)
4	0.500	22"
5	0.625	28"
6	0.750	33"

90° HOOK DIMENSIONS				
BAR SIZE	A	D	LE	
#4	8"	3"	6"	
#5	10"	3 3/4"	7 1/2"	
#6	1'-0"	4 1/2"	9"	

180° HOOK DIMENSIONS				
BAR SIZE	A	J	D	LE
#4	6"	4"	3"	2 1/2"
#5	7"	5"	3 3/4"	2 1/2"
#6	8"	6"	4 1/2"	3"



STANDARD HOOK DETAILS

CONCRETE FOOTING SCHEDULE						
TYPE	DEPTH	WIDTH	LENGTH	REINFORCEMENT*		REMARKS
				LONGITUDINAL	TRANSVERSE	
F2.5	0' - 10"	2' - 6"	2'-6"	(3) - #4	(3) - #4	-

KELSO PETERBILT LEAN-TO
2408 TALLEY WAY
KELSO, WA

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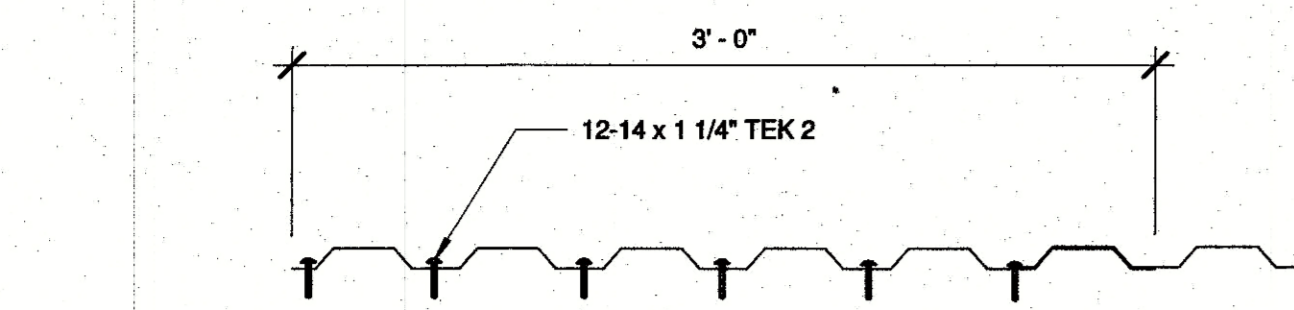
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DRAWN BY: MDG	BY:
DATE:	REVISIONS:



TETON STRUCTURAL ENGINEERS
A PROFESSIONAL CORPORATION
136 SOUTH STATE STREET SHELLEY, IDAHO 83274
PHONE (208)-357-2420 FAX (208)-357-2419

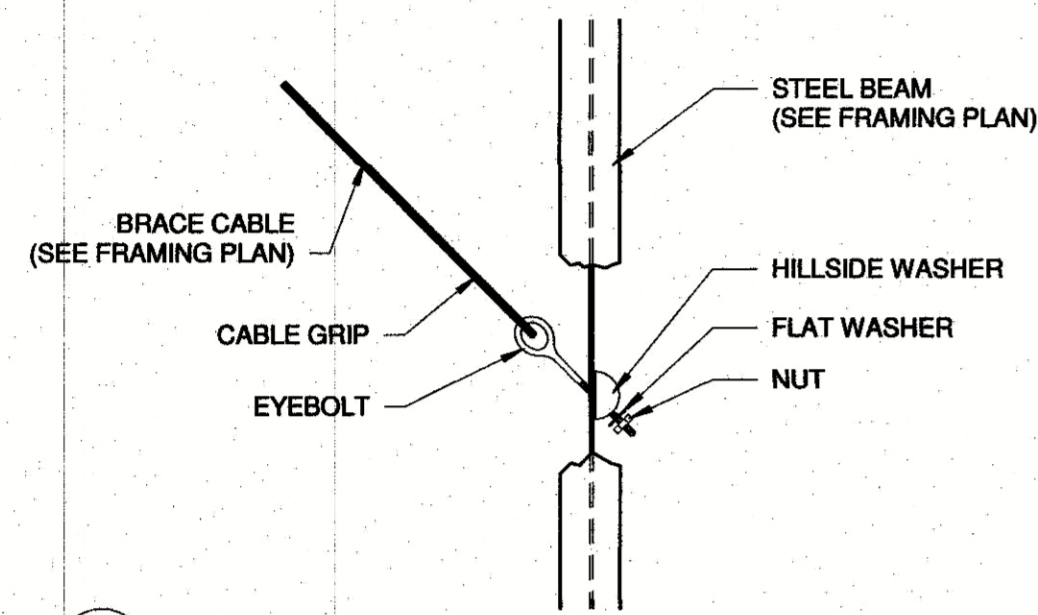
PROJECT NO. 221338
SHEET NO. S1
OF TWO

JAN 13 2022

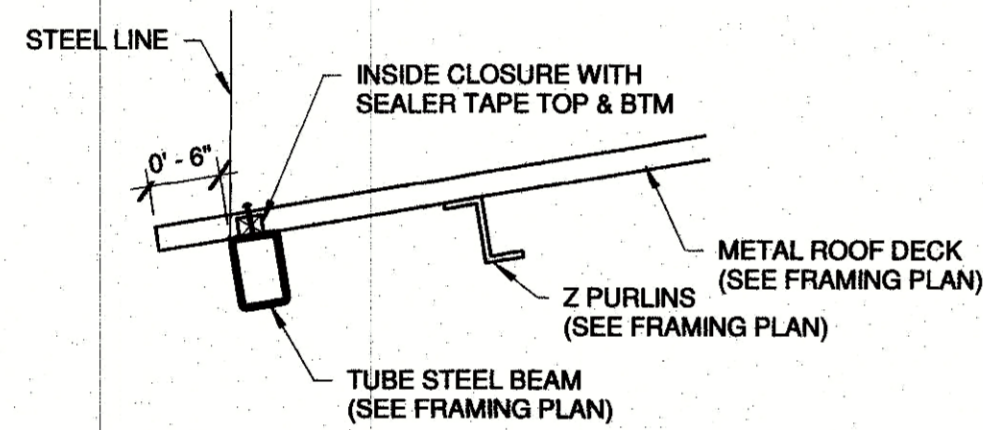


SIDE LAP FASTENERS #10 TEK SCREWS @ 2'-0" O.C.
FASTENER SPACING @ EAVE, END LAPS, AND HIGH LEAVE

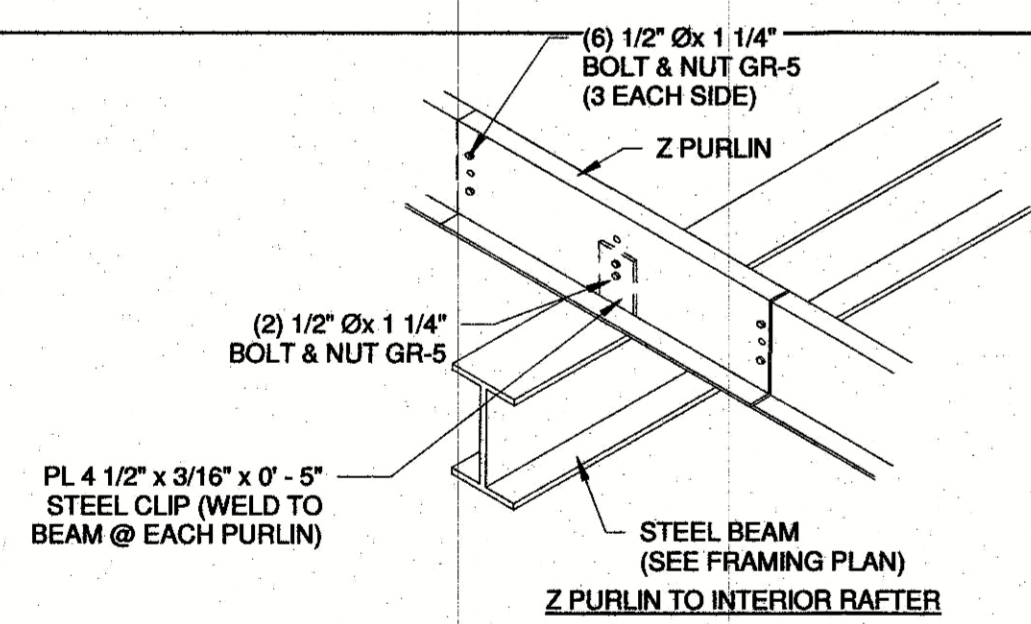
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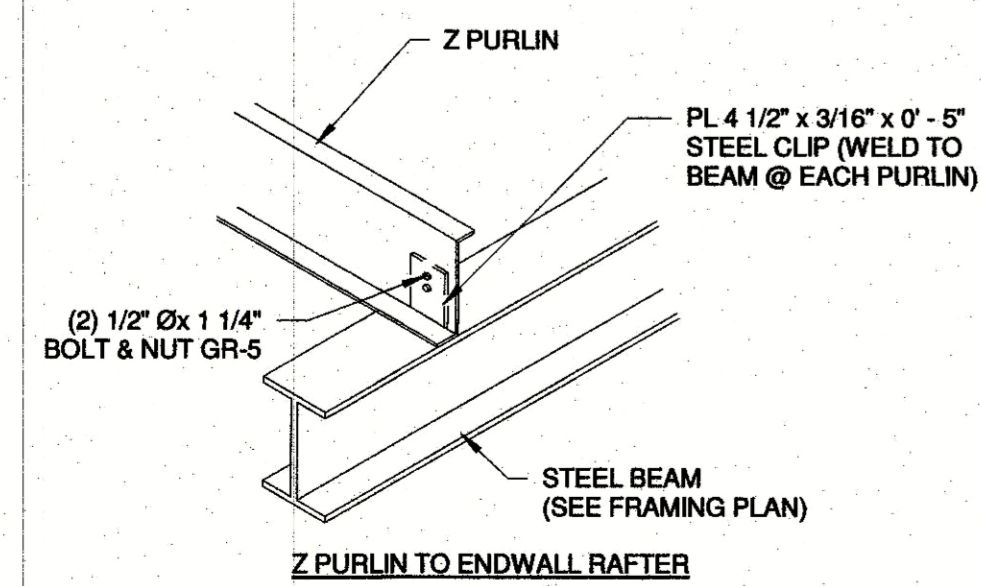
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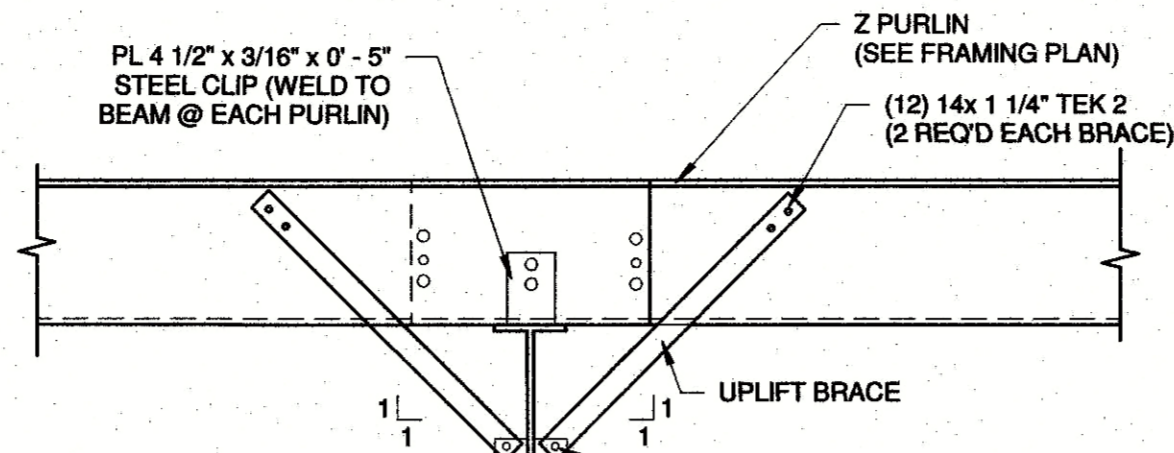
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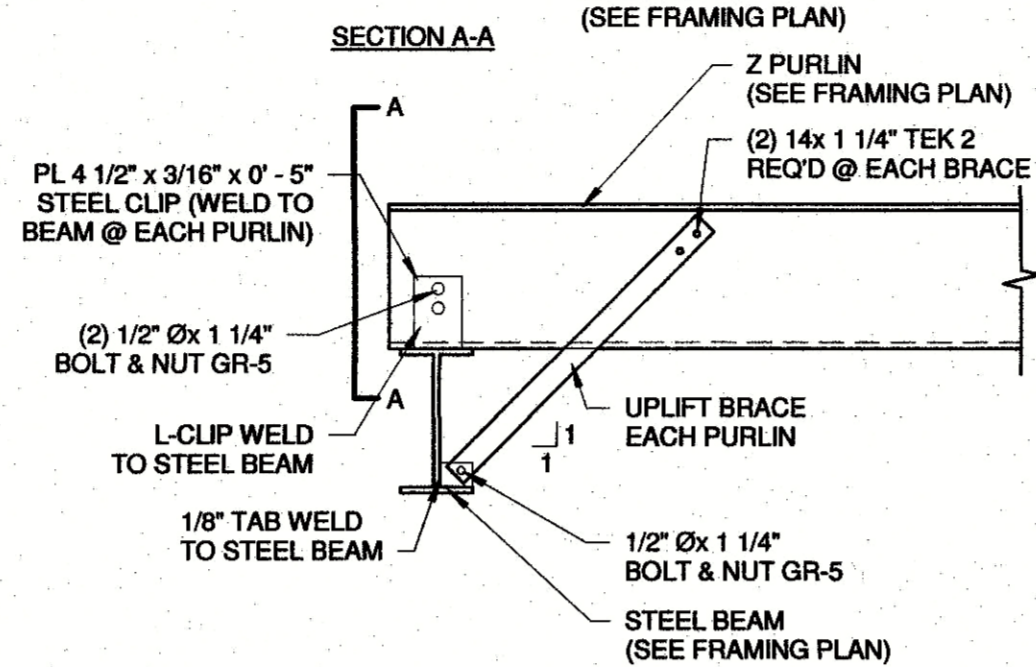
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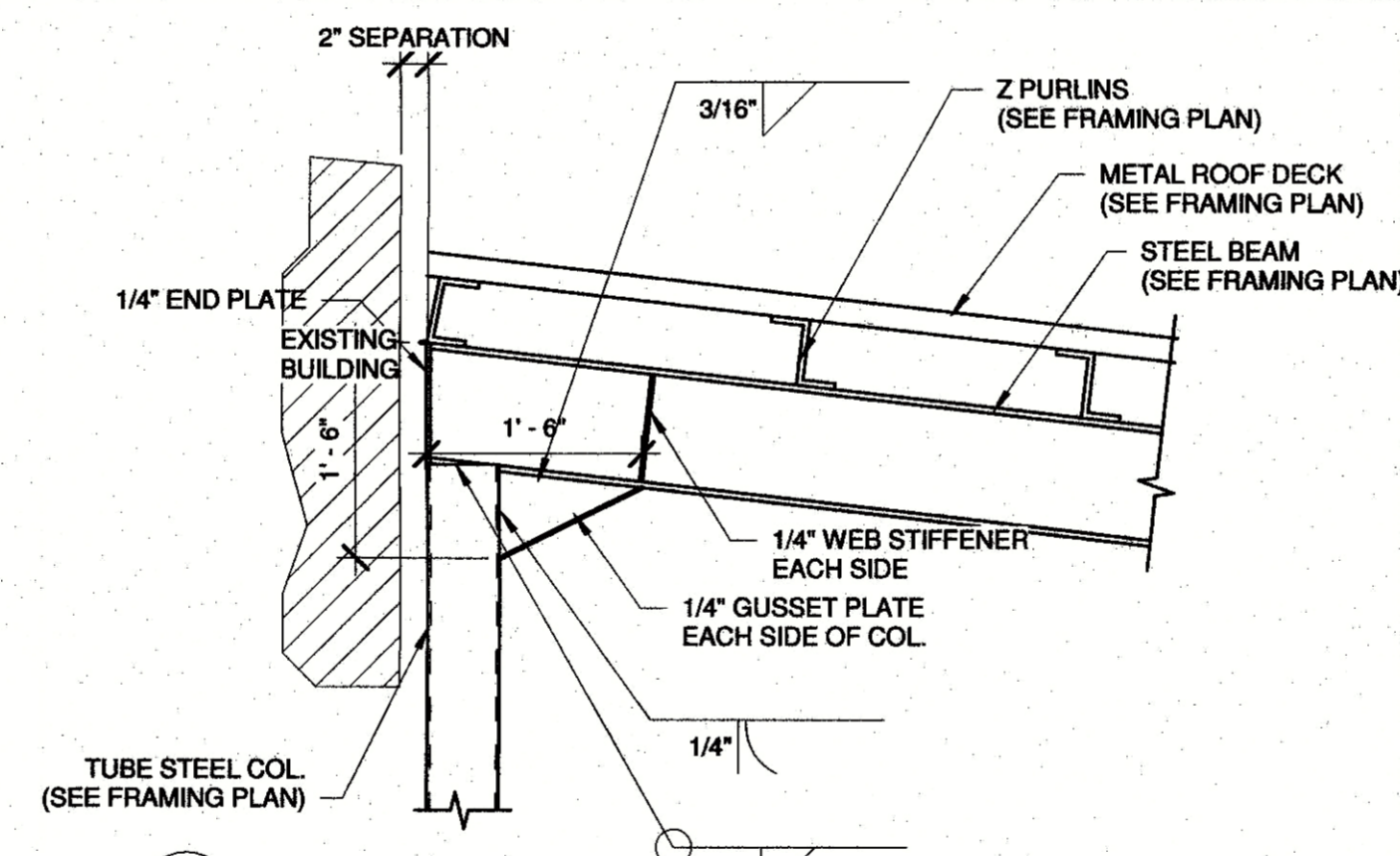
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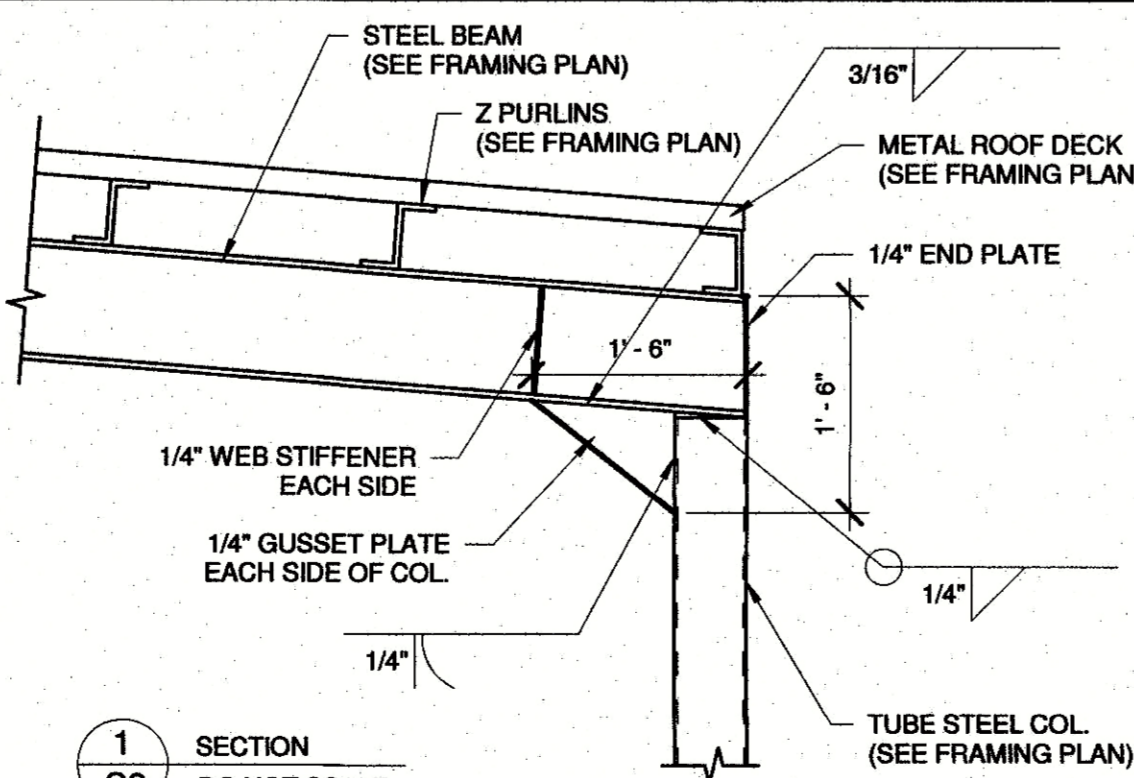
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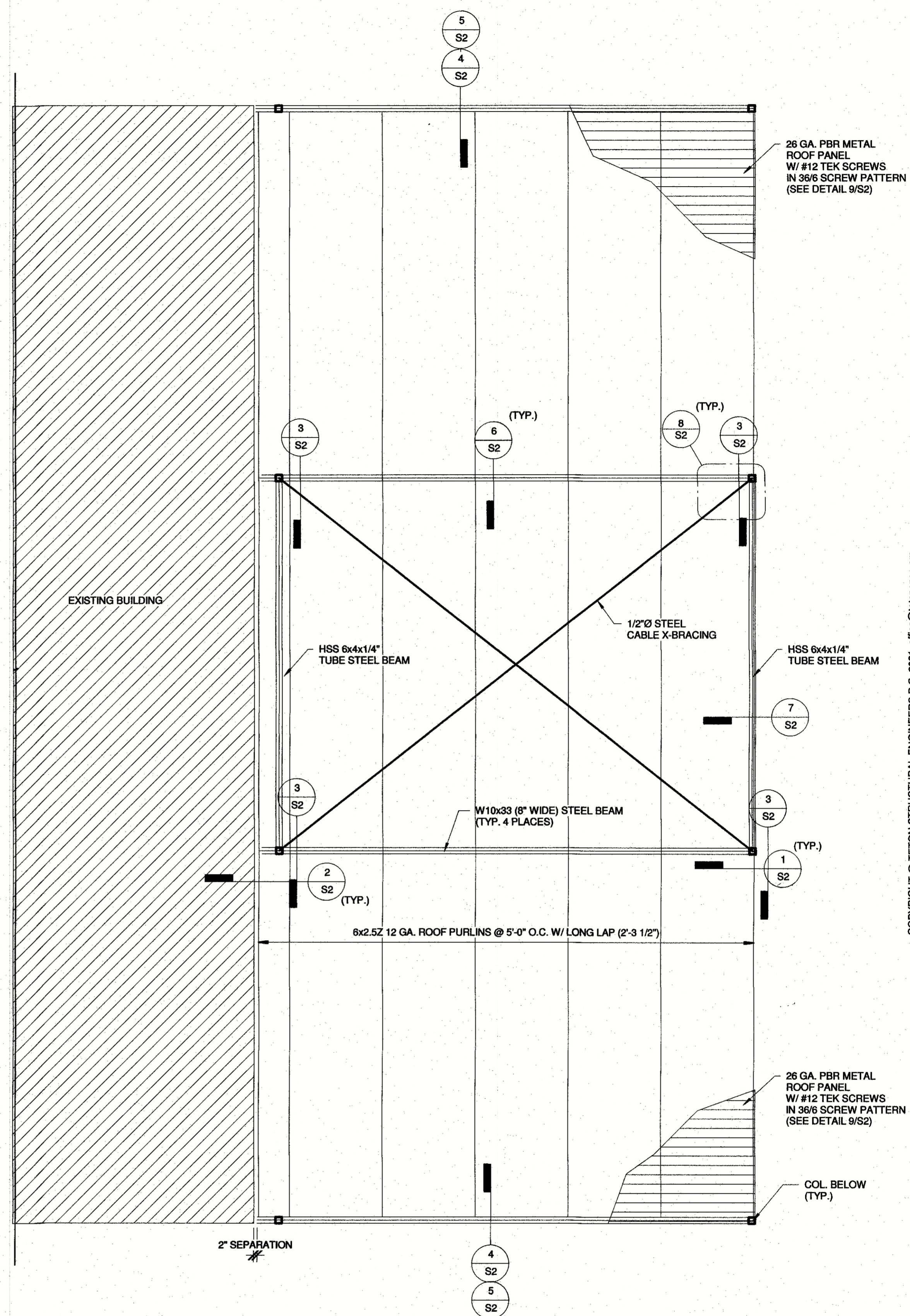
3 SECTION
S2 DO NOT SCALE



2 SECTION
S2 DO NOT SCALE



1 SECTION
S2 DO NOT SCALE

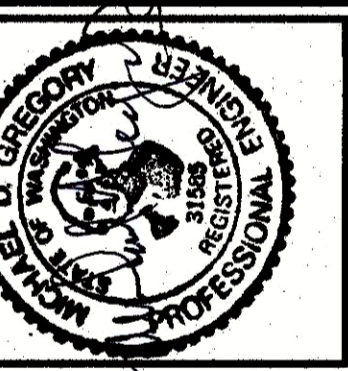


ROOF FRAMING PLAN
1/4" = 1'-0"

KELSO PETERBILT LEAN-TO
2408 TALLEY WAY
KELSO, WA

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REVISIONS	BY
DATE	



TETON
STRUCTURAL ENGINEERS
A PROFESSIONAL CORPORATION
136 SOUTH STATE STREET SHELLEY, IDAHO 83274
PHONE (208)-357-2420 FAX (208)-357-2419

PROJECT NO.
221338
SHEET NO.
S2
OF TWO

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JAN 13 2022

FOUNDATION NOTES

1. Design Information and Loads

- A. Foundation design is in accordance with 2018 International Building Code using the reactions provided by Nucor Building Systems Group for the following design criteria:
- B. Roof Live Load 20 psf
 C. Ground Snow Load 25 psf
 Roof Snow Load 17.5 psf
 Minimum Uniform Roof Snow Load 25 psf
 Exposure Factor 1.0
 Importance Factor 1.0
- D. Wind
 Wind Speed 115 mph
 Exposure C
 Risk Category II
- E. Seismic Information
 S_s 0.895 S₁ 0.431
 S_{ps} 0.716 S₀₁ 0.537
 Site Class D
 Seismic Design Category D
 Importance Factor 1.0
 Analysis Procedure Equivalent Lateral Force Method
 Basic SFRS OSMF & OSCBF
 F. Frost Depth 6"

2. Earthwork

- A. Foundation Design Values (assumed)
 i. Allowable Soil Bearing Pressure - 1500 psf
 ii. Coefficient of Friction - 0.25
 iii. Passive Earth Pressure - 200 psf/ft of depth
- B. The building pad area shall be stripped of all frozen soil, debris, vegetation, and topsoil. All fill soils and any remaining loose natural soils shall be excavated to expose suitable natural soils.
- C. Proof roll the entire building pad area to locate and remove all soft spots. Replace with compacted structural fill.
- D. Place all footings and slabs on undisturbed natural soil or on properly compacted structural fill. Contractor shall verify that soil under footings is suitable to support footings.
- E. Structural Fill: Structural fill should consist of well-graded sandy gravels with a maximum particle size of 3 inches and 5 to 15 percent fines (materials passing the No. 200 sieve). The liquid limit of fines should not exceed 35 and the plasticity index should be below 15. All fill soils should be free from topsoils, highly organic material, frozen soil, and other deleterious materials. Structural fill should be placed in maximum 8-inch thick loose lifts at a moisture content within 2 percent of optimum and compacted to at least 95 percent of modified proctor density (ASTM D1557) under the building and 90 percent under concrete flatwork.

F. It is the responsibility of the contractor to ensure that the depth of the bottom of the foundation is far enough below the adjacent grade to ensure adequate frost protection.

3. Concrete and Reinforcement

- A. Material Standards
 i. Concrete
 a. Footings: Exposure Classes F0, S0, W0, C0
 f'c = 3000 p.s.i., max. w/cm ratio = 0.55
 b. Exterior Walls: Exposure Classes F1, S0, W0, C1
 f'c = 3500 p.s.i., max. w/cm ratio = 0.55
 c. Interior Walls: Exposure Classes F0, S0, W0, C0
 f'c = 3000 p.s.i., max. w/cm ratio = N.A.
 d. Interior Slabs: Exposure Classes F0, S0, W0, C0
 f'c = 3500 p.s.i., max. w/cm ratio = 0.55
 e. Air content for Exposures F1-F3 must meet the requirements of Table 19.3.3.1 of ACI 318-14. Air-entraining admixtures shall conform to ASTM C260
 f. The cement type for Exposures S1-S3 must meet the requirements of Table 19.3.2.1 of ACI 318-14. Cement shall conform to ASTM C150
 g. Calcium Chloride admixture shall not be used in Exposures S2 and S3
- ii. Reinforcing
 a. Rebar - ASTM A615 Grade 60 (Fy = 60 ksi)
 b. Welded wire - ASTM A1064
 c. Epoxy/Adhesive - Simpson SET-XP (ICC-ES ESR-2508), Hilti RE-500V3 (ICC-ES ELC-3814), or Dewart Pure10+ (ICC-ES ESR-3298) unless noted otherwise in the drawings.
- iii. Anchor Rods/Bolts
 a. All anchor rods shall be cast-in-place headed anchor rods. Use of post-installed (epoxy, adhesive, expansion, screw, etc.) anchors is not allowed without written permission from MVE or unless specifically noted in the drawings.
 b. Steel column anchor rods/bolts - ASTM F1554 Grade 55 with ASTM A563 heavy hex nuts and hardened washers (unless noted otherwise)
 c. Wood framing anchors - ASTM A307 with A36 plate washers
 d. Headed stud anchors (HSA) - ASTM A108
 e. Deformed bar anchors (DBA) - ASTM A496
 f. Screw Anchors for jombs as indicated in the typical anchor rod schedule - Simpson Titen HD (ICC-ES ESR-2713), Hilti Kwik HUS-TZ (ICC-ES ESR-3027), or Dewalt Screwbolt+ (ICC-ES ESR-2526)

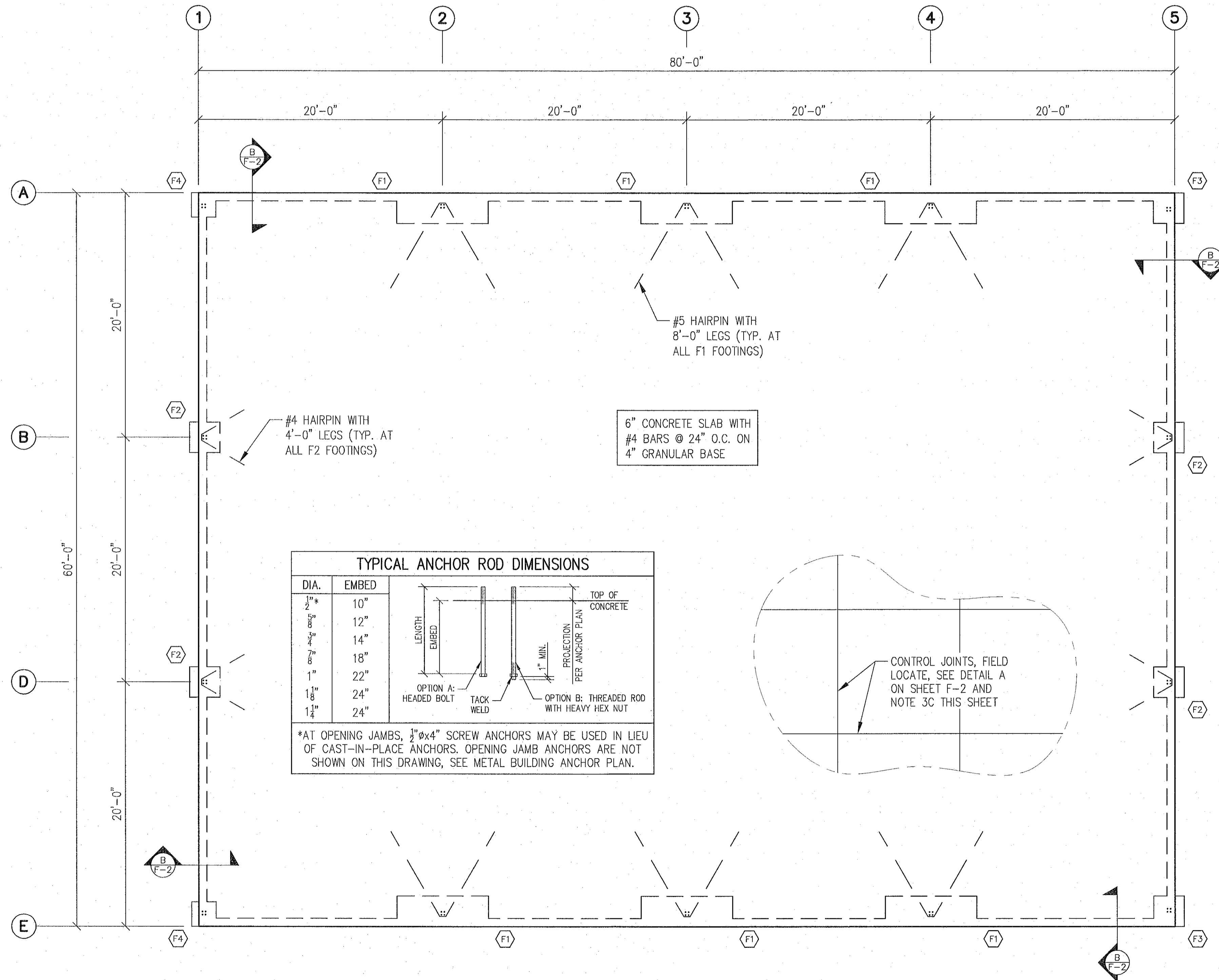
- g. Use of hooked anchor rods/bolts is limited under the ACI and the IBC. Headed anchor rods/bolts must be used where indicated in the details.
- h. The symbols \odot , A.R./ \odot , A.B. as shown in the drawings indicate the center line of the anchor rod/bolt pattern, not the center line of any individual anchor rod/bolt.
- B. Detail reinforcing to comply with ACI 315 "Manual of Standard Practice for Detailing Reinforcing Concrete Structures" and the Concrete Reinforcing Steel Institute (CRSI) recommendations.
- i. Minimum clear concrete cover for reinforcement shall be as follows unless noted otherwise:
 a. Concrete cast directly against and permanently exposed to earth - 3"
 b. Concrete exposed to weather or earth:
 1. #5 bars or smaller - 1 1/2"
 2. #6 bars or larger - 2"
 c. Concrete not exposed to weather or in contact with the ground - 3/4"
 d. Slabs on grade - as shown in details, 3/4" min. from top of slabs not exposed to weather
- ii. Lap Splice Lengths with 1 1/2" minimum clear cover
 a. f'c = 2500-3500 p.s.i.
 1. #6 and smaller - 49 bar diameters
 2. #7 and larger - 76 bar diameters
 b. f'c = 4000 p.s.i. or greater
 1. #6 and smaller - 38 bar diameters
 2. #7 and larger - 60 bar diameters
 c. Increase lap splice lengths by 50% where epoxy coated bars are used.
- iii. Stagger splices in walls so that no two adjacent bars are spliced in the same location, unless shown otherwise.
- iv. Make all bars continuous around corners or provide corner bars of equal size and spacing.
- v. Where 12 inches or less of fresh concrete is placed below horizontal reinforcing lap splice length may be reduced by 30%.
- vi. Vertical bars in walls, grade beams, and piers to terminate in footings with ACI standard hooks (12 bar diameters) to within 4" of the bottom of the footing unless noted otherwise.
- vii. Horizontal wall reinforcing shall terminate at the ends of walls with a 90 degree hook plus a 6 bar diameter extension, unless shown otherwise.
- viii. Horizontal wall reinforcing shall be continuous through construction and control joints.
- ix. Splices in horizontal reinforcing shall be staggered. Splices in two curtains (where used) shall not occur in the same location.
- x. Use chairs or other support devices as required for proper clearance.
- xi. Rebar hairpins shall be centered in slabs and shall be wire tied to the slab reinforcing (if any). Rebar hairpins shall be continuous through walls and piers; lap splices in hairpins may only occur in the floor slab unless noted otherwise.
- C. Control joints in slabs on grade are recommended to control cracking. See plans for control joint spacing and details.
- D. Slabs and grade beams shall not have joints in a horizontal plane. All reinforcement shall be continuous through all construction joints.
- E. Floor slab thickness and reinforcing shown in these drawings are adequate to support typical uniform loads only. Mountain View Engineering has not designed the slab for any specific concentrated forces such as those from vehicles, storage racks, or heavy equipment (unless noted otherwise).
- F. Welding of rebar is not allowed unless specifically indicated in the drawings. All embedments, reinforcing, and dowels shall be securely tied to framework or to adjacent reinforcing prior to placement of the concrete. Tack welding of rebar joints in grade beams, walls, or cages is not allowed. Where welding of rebar is shown in the drawings, all rebar to be welded shall be ASTM A706 Grade 60.

4. Special Inspections

- A. Concrete
 i. Spot Footings - Not required (IBC 1705.3 Exception 1)
 ii. Continuous Ftgs. - Not required (IBC 1705.3 Exception 2.3)
 iii. Slabs - Not required (IBC 1705.3 Exception 3)
 iv. Grade Beams - Not required (IBC 1705.3 Exception 4)
 v. Walls - Not required (IBC 1705.3 Exception 4)
 vi. Anchor rods/bolts - Required (IBC Table 1705.3) Special inspection may be waived subject to the approval of the building official.
- B. Steel Reinforcement
 i. Placement - Third party special inspection of reinforcing placement need only be performed where specifically required by the building official.
 ii. Welding - Special inspection of rebar welding is required (if any is used).

5. Miscellaneous

- A. The contractor shall notify engineer of any variations in dimensions.
 B. The engineer is not responsible for any deviations from these plans unless such changes are authorized in writing by the engineer.



(F1) INDICATES APPLICABLE FOOTING DETAIL.

FOUNDATION PLAN
 SCALE: 3/16"=1'-0"

NOTE: COORDINATE THIS DRAWING WITH THE NUCOR BUILDING SYSTEMS ANCHOR BOLT PLAN.

MOUNTAIN VIEW ENGINEERING, INC.
 Consulting
 Structural Engineering
 345 North Main Street Ste. A, Brigham City, Utah 84302 (435) 734-9700 Fax (435) 734-9519

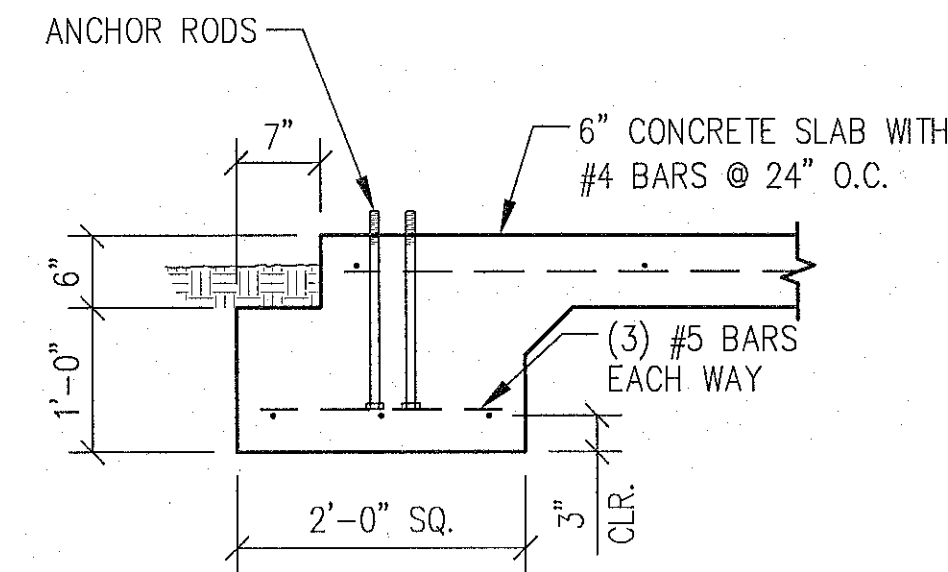
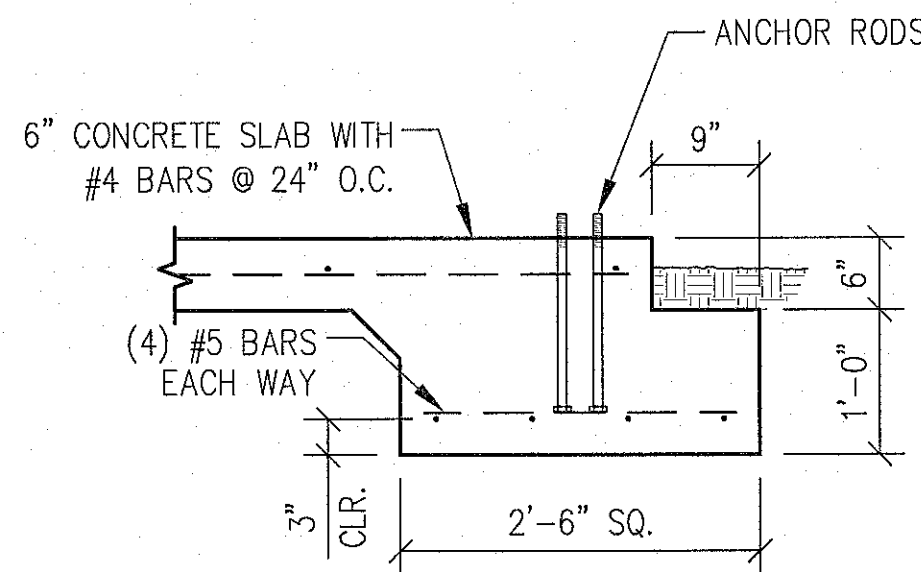
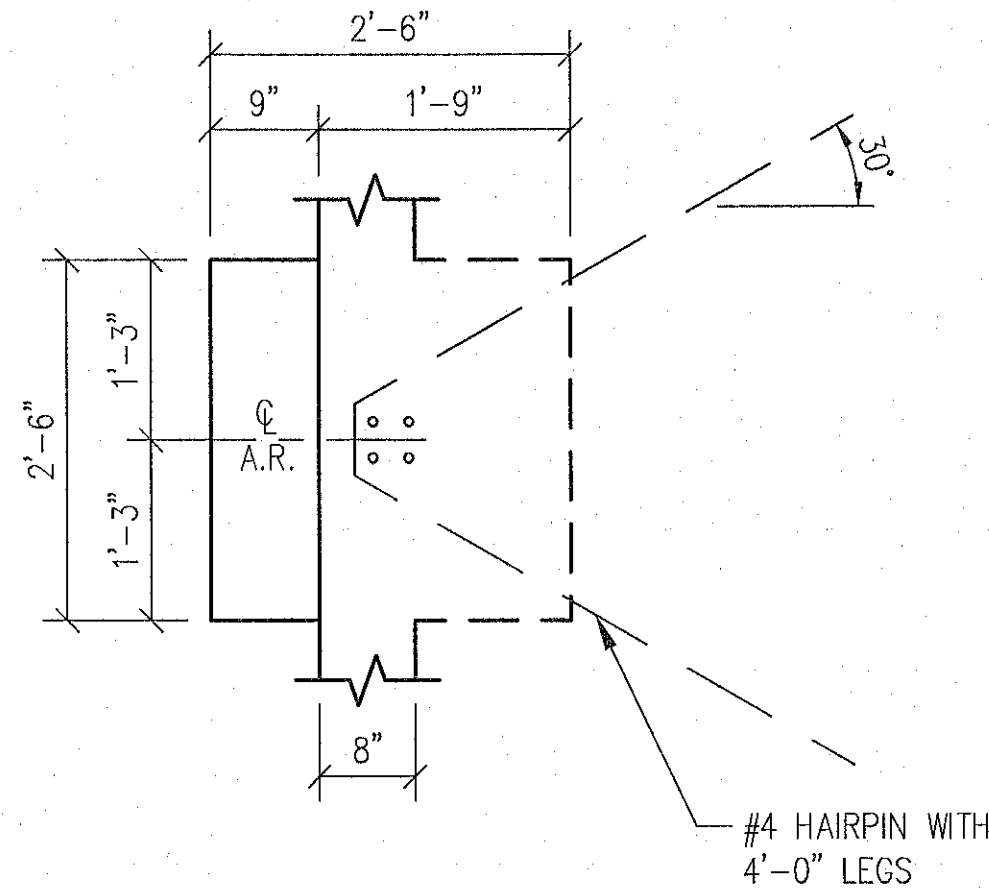
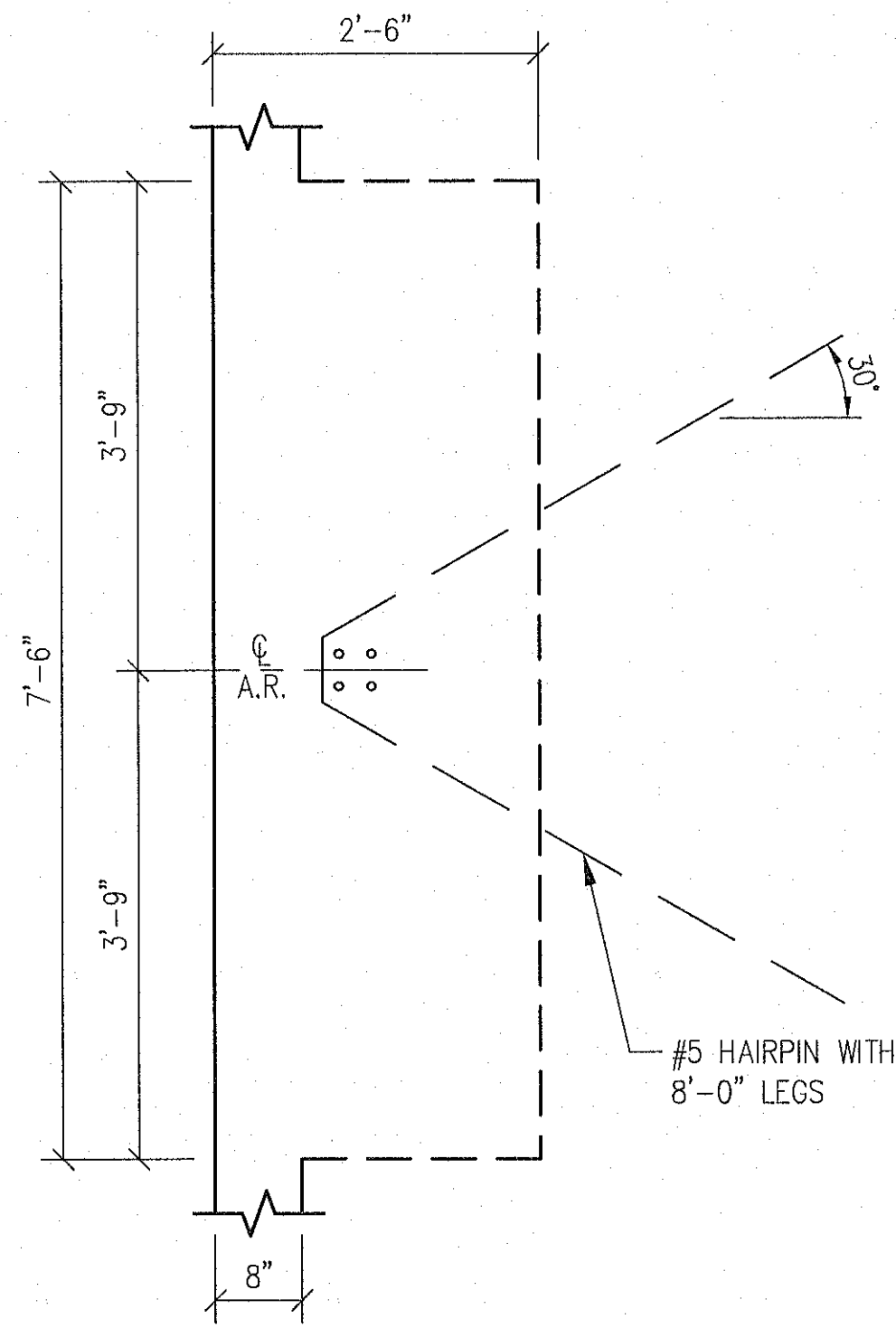
FOUNDATION PLAN
 SHEET TITLE:
 JOB NAME: **KELSO PETERBLT 60X80**
 LOCATION: **KELSO, WASHINGTON**
 CONTRACTOR: **ULTIMATE STEEL ERECTION**

DATE	BY	DESCRIPTION
12-1-21	N.H.	FOR PERMIT

WORLD WALLACE
 STATE OF WASHINGTON
 PROFESSIONAL ENGINEER
 DEC 01 2021

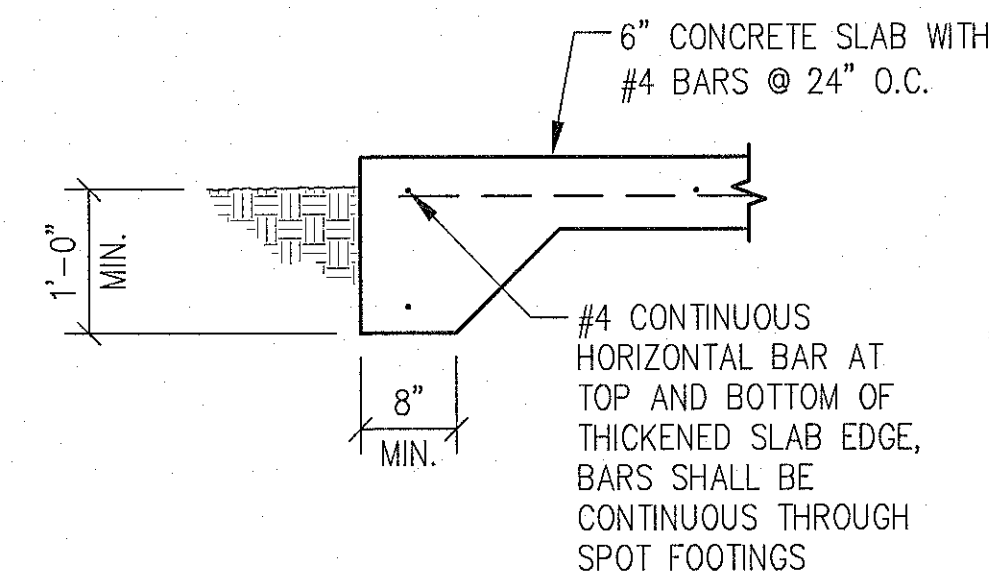
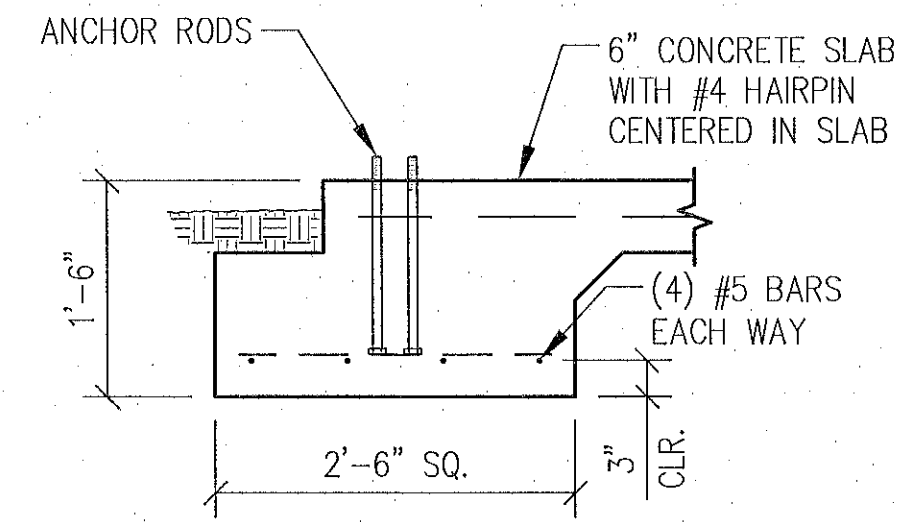
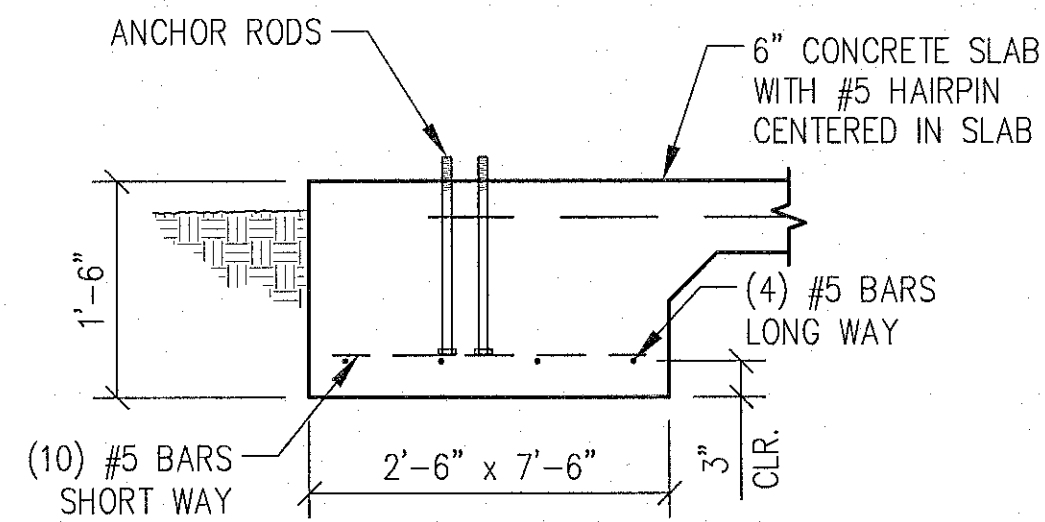
SHEET NUMBER:
F-1

DRAWN BY: N.H.
 ENGINEER: J. LARSEN
 MVE JOB NUMBER: **211599**



F3 CORNER FOOTING DETAIL
3/4"=1'-0"

F4 CORNER FOOTING DETAIL
3/4"=1'-0"



F1 SIDEWALL FOOTING DETAIL
3/4"=1'-0"

F2 ENDWALL FOOTING DETAIL
3/4"=1'-0"

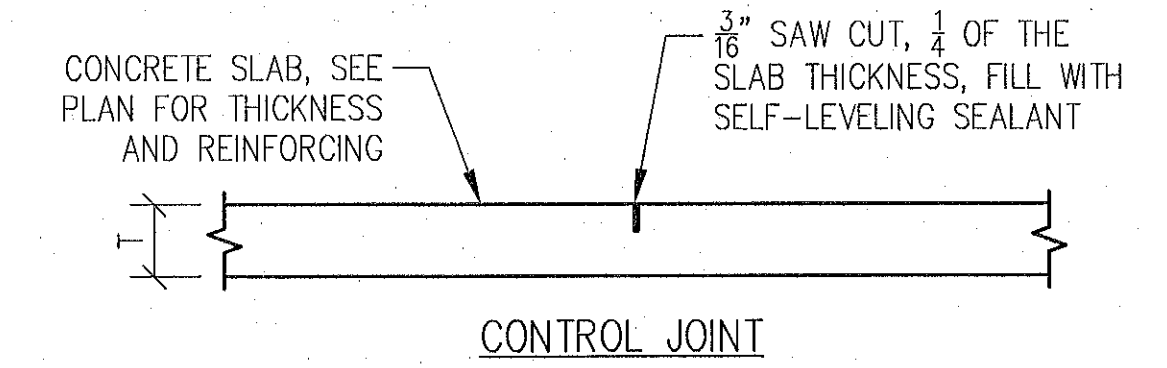
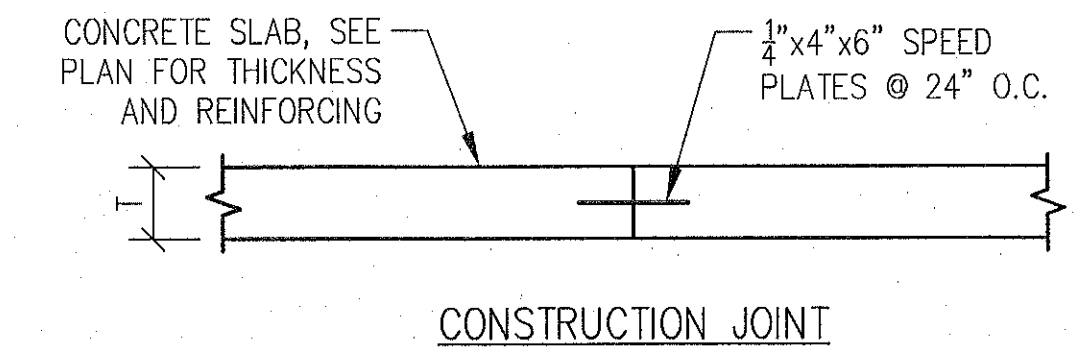
B THICKENED SLAB EDGE DETAIL
3/4"=1'-0"

CONTROL JOINT NOTES:

- Control joints shall be field located by the contractor.
- Control joints shall be located to limit the frequency and width of random cracks in the concrete slab.
- Locate and install control joints in accordance with ACI 360R "Design of Slabs on Ground" and the details shown.
- Maximum spacing of joints shall be per the table below.
- Saw cuts should be made as soon as possible.

MAXIMUM SPACING OF CONTROL JOINTS

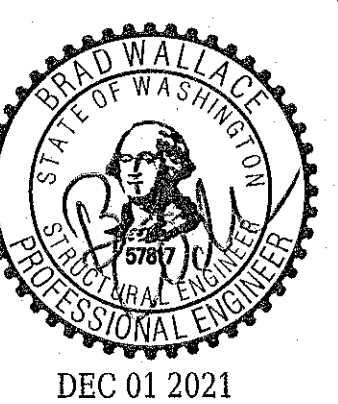
Slab thickness (T), in.	Slump 4 in. to 6 in.	
	Maximum-size aggregate less than 3/4 in.	Maximum-size aggregate 3/4 in. and larger
4	8 ft.	10 ft.
5	10 ft.	13 ft.
6	12 ft.	15 ft.
7	14 ft.	18 ft.
8	16 ft.	20 ft.



A CONTROL JOINTS
N.T.S.

PLAN ISSUE DATES

DATE	BY	DESCRIPTION
12-1-21	N.H.	FOR PERMIT

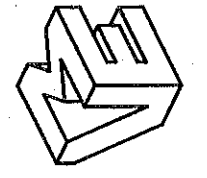


SHEET NUMBER:

F-2

DRAWN BY: N.H.
ENGINEER: J. LARSEN
MVE JOB NUMBER: **211599**

SHEET TITLE: FOUNDATION DETAILS
JOB NAME: KELSO PETERBILT 60X80
LOCATION: KELSO, WASHINGTON
CONTRACTOR: ULTIMATE STEEL ERECTION



MOUNTAIN VIEW ENGINEERING, INC.

Structural Engineering Consulting Design
345 North Main Street Ste. A, Brigham City, Utah 84302 (435) 734-9700 Fax (435) 734-9519